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INFLUENCE OF THE HARDENING CHARACTERISTICS OF MATERIAL ON THE CRITICAL LOAD IN THE ELASTO - PLASTIC STABILITY PROBLEM OF CONICAL SHELLS

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Abstract. In this paper by using the theory of elasto-plastic processes and adjacentequilibrium criterion the governing equations of the elasto-plastic stability problem of conical shells are derived. The Bubnov-Galerkin's method combined with the loading parameter method are applied in solving the mentioned problem. The influence of the hardening characteristics of material on the critical load is investigated.

1. INTRODUCTION

Analysis of the elasto-plastic stability problem of shells with homogeneous membrane stress of the prebuckling state was considered by some authors $[1 \div 4]$, but in the case when the prebuckling state is non-homogeneous many difficulties arise in solving the problem, because now stability equations are a set of partial differential equations with variable coefficients. Otherwise we can get more difficulties in determination of material functions occured in the constitutive relations, for example the secant modulus and tangent modulus which become functions of point coordinates. Furthermore in the shell occur elastic and plastic zones, the boundary of which is unknown, it must be determined simultaneously in the solution process.

In this paper the governing equations of the elasto-plastic stability problem of conical shells are developed based on the theory of elasto-plastic processes and the adjacent-equilibrium criterion. The Bubnov-Galerkin's method combined with the loading parameter method can be applied in solving considered problem. A piece-wise linearization procedure of the material function figured in constitutive relations is demonstrated for material with general hardening characteristics and the influence of this characteristics on the critical load is investigated.

2. PREBUCKLING STATE OF A CONICAL SHELL.

Points in the middle surface of a conical shell may be referred to coordinates (x, θ) , where x is a coordinate taken from the shell top to the considered point in the generatrix direction, θ - a circumferential coordinate, α - an open angle at the shell top, *l*- the length of shell anh *h*- the shell thickness. The stress occurs in the prebuckling state depending on loading process, here we restrict ourselves the applied load is axisymmetric and the linear bending equations are used for the prebuckling deformation. If the shell is acted on by external pressure with intensity *p*, the prebuckling stress state is of the form

$$\sigma_x^0 = -\frac{px}{2h} \operatorname{tg} \alpha, \quad \sigma_\theta^0 = -\frac{px}{h} \operatorname{tg} \alpha, \quad \sigma_{x\theta}^0 = 0, \quad \sigma_u^0 = \frac{\sqrt{3} px}{2h} \operatorname{tg} \alpha;$$

$$N_x^0 = -\frac{px}{2} \operatorname{tg} \alpha, \quad N_\theta^0 = -px \operatorname{tg} \alpha, \quad N_{x\theta}^0 = 0.$$

It is clear that along the shell generatrix the stress intensity increases linearly with respect to x. Thus at points $0 \le x \le x_s$ elastic state occurs, while $x_s \le x \le l$ the shell is in plastic state, the boundary of two zones is determined by

$$x_s = \frac{2\,\sigma_s\,h}{\sqrt{3}\,p\,\mathrm{tg}\,\alpha},$$

where σ_s - yield stress (when $\sigma_u^0 = \sigma_s$), x_s - elastic- plastic zones boundary. This boundary will be determined simultaneously in the solving process. Putting

$$t = \frac{x}{l}, \quad \sigma_u^0 = \frac{\sqrt{3} p l}{2 h} \operatorname{tg} \alpha \frac{x}{l} = \sigma_M^0 t,$$

where

$$\sigma_M^0 = \frac{\sqrt{3}\,p\,l}{2\,h}\,\mathrm{tg}\,\alpha_{\rm s}$$

one can get

$$0 \leq t \leq 1, \quad \sigma_s = \sigma_M^0 t_s, \quad t_s = \frac{x_s}{l} = \frac{\sigma_s}{\sigma_M^0}.$$

3. STABILITY EQUATIONS

Applying stability equations [5] for conical shells subjected to external pressure yields

$$R_2 \to \infty$$
, $\lim_{R_2 \to \infty} R_2 d\varphi = dx$, $r = x \sin \alpha$,
 $\varphi = \pi/2 - \alpha$, $\sin \varphi = \cos \alpha$, $\cos \varphi = \sin \alpha$,

$$\frac{\partial}{\partial x}(x\,\delta N_x) + \frac{1}{\sin\alpha}\frac{\partial\delta N_{x\theta}}{\partial\theta} - \delta N_{\theta} = 0,$$

$$\frac{1}{\sin\alpha}\frac{\partial\delta N_{\theta}}{\partial\theta} + \frac{1}{x}\frac{\partial}{\partial x}(x^2\,\delta N_{x\theta}) = 0,$$

$$\frac{\partial^2}{\partial x^2}(x\,\delta M_x) + \frac{2}{\sin\alpha}\left(\frac{\partial^2\delta M_{x\theta}}{\partial x\partial\theta} + \frac{1}{x}\frac{\partial\,\delta M_{x\theta}}{\partial\theta}\right) + \frac{1}{x\,\sin^2\alpha}\frac{\partial^2\delta M_{\theta}}{\partial\theta^2} - \frac{\partial\,\delta M_{\theta}}{\partial\theta^2} - \frac{\partial\,\delta M_{\theta}}{\partial x} - \delta N_{\theta}\cot g\,\alpha + p\,tg\,\alpha\left(x\frac{\partial\,\delta w}{\partial x} + \frac{x^2}{2}\frac{\partial^2\,\delta w}{\partial x^2} + \frac{1}{\sin^2\alpha}\frac{\partial^2\,\delta w}{\partial\theta^2}\right) = 0,$$
(3.1)

with the following boundary conditions: - at the fixed point 0 of the shell top

$$\delta u = \delta v = \delta w = 0 \quad \text{with} \quad x = 0; \tag{3.2}$$

- the end cross section of the shell is simply supported such that

$$\delta w = 0, \quad \delta v = 0, \quad \delta M_x = 0, \quad \delta N_x = 0. \tag{3.3}$$

Remark:

• With $\alpha = \pi/2$ equations (3.1) reduce to stability equations for a circular plate.

• With $x \sin \alpha = a = const$ and $\alpha = 0$, equations (3.1) become stability equations for a circular cylindrical shell of radius a.

According to the elasto-plastic process theory [3] the expressions for internal forces increments and internal moments increments of a conical shell subjected to external pressure are obtained as follows

$$\delta N_{x} = h \Big[\Big(N + \frac{\Phi'}{3} \Big) \frac{\partial \delta u}{\partial x} + \frac{2}{3} \Phi' \Big(\frac{1}{x \sin \alpha} \frac{\partial \delta v}{\partial \theta} + \frac{\delta u}{x} + \frac{\delta w}{x} \cot g \alpha \Big) \Big],$$

$$\delta N_{\theta} = h \frac{2}{3} \Phi' \Big[\frac{\partial \delta u}{\partial x} + 2\Big(\frac{1}{x \sin \alpha} \frac{\partial \delta v}{\partial \theta} + \frac{\delta u}{x} + \frac{\delta w}{x} \cot g \alpha \Big) \Big],$$

$$\delta N_{x\theta} = h \frac{N}{3} \Big[\frac{\partial \delta v}{\partial x} - \frac{\delta v}{x} + \frac{1}{x \sin \alpha} \frac{\partial \delta u}{\partial \theta} \Big],$$

$$\delta M_{x} = -\frac{h^{3}}{12} \Big[\Big(N + \frac{\Phi'}{3} \Big) \frac{\partial^{2} \delta w}{\partial x^{2}} + \frac{2}{3} \Phi' \Big(\frac{1}{x^{2} \sin^{2} \alpha} \frac{\partial^{2} \delta w}{\partial \theta^{2}} + \frac{1}{3} \frac{\partial \delta w}{\partial x} \Big) \Big],$$

$$\delta M_{\theta} = -\frac{h^{3}}{18} \Phi' \Big[\frac{\partial^{2} \delta w}{\partial x^{2}} + 2\Big(\frac{1}{x^{2} \sin^{2} \alpha} \frac{\partial^{2} \delta w}{\partial \theta^{2}} + \frac{1}{x} \frac{\partial \delta w}{\partial x} \Big) \Big],$$

$$\delta M_{x\theta} = -\frac{h^{3}}{18} N \Big(\frac{1}{x \sin \alpha} \frac{\partial^{2} \delta w}{\partial x \partial \theta} - \frac{1}{x^{2} \sin \alpha} \frac{\partial \delta w}{\partial \theta} \Big),$$

(3.4)

where $N = \sigma_u^0/s = \Phi(s)/s$ plays a role of a secant modulus and $\Phi'(s)$ - tangent modulus of the hardening material. In this case N and Φ' are functions of x alone, $\delta u, \delta v, \delta w$ are displacement increments in generatrix, circumferential and normal to the shell directions, respectively. Since then without mistakes we can omit the word " δ " in the denotation of increment quantities.

Substituting (3.4) into (3.1) leads to equations

$$\begin{aligned} x(3N+\Phi')\frac{\partial^2 u}{\partial x^2} + (3N+\Phi'+3x\frac{\partial N}{\partial x}+x\frac{\partial \Phi'}{\partial x})\frac{\partial u}{\partial x} + (2x\frac{\partial \Phi'}{\partial x}-4\Phi')\frac{u}{x} + \\ &+\frac{N}{x\sin^2\alpha}\frac{\partial^2 u}{\partial \theta^2} + (N+2\Phi')\frac{1}{\sin\alpha}\frac{\partial^2 v}{\partial x\partial \theta} - (N+4\Phi'-2x\frac{\partial \Phi'}{\partial x})\frac{1}{\sin\alpha}\frac{\partial v}{\partial \theta} + \\ &+2\Phi'\cos\alpha\frac{\partial w}{\partial x} - (4\Phi'-2x\frac{\partial \Phi'}{\partial x})\cot\alpha\frac{w}{x} = 0; \\ xN\frac{\partial^2 v}{\partial x^2} + (N+x\frac{\partial N}{\partial x})\frac{\partial v}{\partial x} - (N+x\frac{\partial N}{\partial x})\frac{v}{x} + 4\frac{\Phi'}{x\sin^2\alpha}\frac{\partial^2 v}{\partial \theta^2} + (N+2\Phi')\frac{1}{\sin\alpha}\frac{\partial^2 u}{\partial x\partial \theta} + \\ &+ (N+4\Phi'+x\frac{\partial N}{\partial x})\frac{1}{x\sin\alpha}\frac{\partial u}{\partial \theta} + 4\frac{\Phi'}{x\sin\alpha}\cot\alpha\frac{\partial w}{\partial \theta} = 0; \\ x(3N+\Phi')\frac{\partial^4 w}{\partial x^4} + (6N+2\Phi'+6x\frac{\partial N}{\partial x}+2x\frac{\partial \Phi'}{\partial x})\frac{\partial^3 w}{\partial x^3} + \\ &+ (3x\frac{\partial^2 N}{\partial x^2}+6\frac{\partial N}{\partial x}-4\frac{\Phi'}{x}+4\frac{\partial \Phi'}{\partial x}+x\frac{\partial^2 \Phi'}{\partial x^2})\frac{\partial^2 w}{\partial x^2} + (\frac{4}{x^2}\Phi'-\frac{4}{x}\frac{\partial \Phi'}{\partial x}+2\frac{\partial^2 \Phi'}{\partial x^2})\frac{\partial w}{\partial x} + . \end{aligned}$$

$$+4(N+\Phi')\frac{1}{x\sin^{2}\alpha}\frac{\partial^{4}w}{\partial x^{2}\partial\theta^{2}}-4(N+\Phi'-x\frac{\partial N}{\partial x}-x\frac{\partial \Phi'}{\partial x})\frac{1}{x^{2}\sin^{2}\alpha}\frac{\partial^{3}w}{\partial x\partial\theta^{2}}+$$

$$+4(N+3\Phi'-x\frac{\partial N}{\partial x}-2x\frac{\partial \Phi'}{\partial x}+\frac{x^{2}}{2}\frac{\partial^{2}\Phi'}{\partial x^{2}})\frac{1}{x^{3}\sin^{2}\alpha}\frac{\partial^{2}w}{\partial\theta^{2}}+4\frac{\Phi'}{x^{3}\sin^{4}\alpha}\frac{\partial^{4}w}{\partial\theta^{4}}+$$

$$+\frac{48\Phi'}{h^{2}}\cot g\alpha(\frac{1}{2}\frac{\partial u}{\partial x}+\frac{1}{x\sin\alpha}\frac{\partial v}{\partial\theta}+\frac{u}{x}+\frac{w}{x}\cot g\alpha)+$$

$$-\frac{36p}{h^{3}}tg\alpha(x\frac{\partial w}{\partial x}+\frac{x^{2}}{2}\frac{\partial^{2}w}{\partial x^{2}}+\frac{1}{\sin^{2}\alpha}\frac{\partial^{2}w}{\partial\theta^{2}})=0.$$
(3.5)

4. SOLVING METHOD

a) In the case of elastic conical shells

$$N = \Phi' = 3G,$$

equations (3.5) reduce to ones of elastic stability equations considered in [7].

b) In the case of conical shells made of material with linear hardening characteristics

$$\Phi^{'}(s) = g = const, \quad N = \frac{\sigma_u^0}{s} = \frac{g\sigma_u^0}{\sigma_u^0 - (\sigma_s - g\varepsilon_s)} = g + \frac{gb}{\sigma_u^0 - b},$$

where $b = \sigma_s - g\varepsilon_s$, the set of equations (3.5) can be written as

$$\begin{aligned} x(3N+\Phi')\frac{\partial^{2}u}{\partial x^{2}} + (3N+\Phi'+3x\frac{\partial N}{\partial x})\frac{\partial u}{\partial x} - 4\Phi'\frac{u}{x} + \frac{N}{x\sin^{2}\alpha}\frac{\partial^{2}u}{\partial \theta^{2}} + \\ + (N+2\Phi')\frac{1}{\sin\alpha}\frac{\partial^{2}v}{\partial x\partial\theta} - (N+4\Phi')\frac{1}{\sin\alpha}\frac{\partial v}{\partial\theta} + 2\Phi'\cot \alpha\frac{\partial w}{\partial x} - 4\Phi'\cot \alpha\frac{w}{x} = 0; \\ xN\frac{\partial^{2}v}{\partial x^{2}} + (N+x\frac{\partial N}{\partial x})(\frac{\partial v}{\partial x} - \frac{v}{x}) + 4\Phi'\frac{1}{x\sin^{2}\alpha}\frac{\partial^{2}v}{\partial\theta^{2}} + (N+2\Phi')\frac{1}{\sin\alpha}\frac{\partial^{2}u}{\partial x\partial\theta} + \\ + (N+x\frac{\partial N}{\partial x} + 4\Phi')\frac{1}{x\sin\alpha}\frac{\partial u}{\partial\theta} + 4\Phi'\frac{1}{x\sin\alpha}\cot \alpha\frac{\partial w}{\partial\theta} = 0; \\ x(3N+\Phi')\frac{\partial^{4}w}{\partial x^{4}} + (6N+2\Phi'+6x\frac{\partial N}{\partial x})\frac{\partial^{3}w}{\partial x^{3}} + (3x\frac{\partial^{2}N}{\partial x^{2}} + 6\frac{\partial N}{\partial x} - 4\frac{\Phi'}{x})\frac{\partial^{2}w}{\partial x^{2}} + \\ + 4\frac{\Phi'}{x^{2}}\frac{\partial w}{\partial x} - 4(N+\Phi'-x\frac{\partial N}{\partial x})\frac{1}{x^{2}}\frac{\partial^{3}w}{\sin^{2}\alpha}\frac{\partial^{3}w}{\partial x\partial\theta^{2}} + 4(N+\Phi')\frac{1}{x\sin^{2}\alpha}\frac{\partial^{4}w}{\partial x^{2}\partial\theta^{2}} + \\ + 4(N+3\Phi'-x\frac{\partial N}{\partial x})\frac{1}{x^{3}}\frac{\partial^{2}w}{\partial\theta^{2}} + 4\Phi'\frac{1}{x^{3}}\frac{1}{\sin^{4}\alpha}\frac{\partial^{4}w}{\partial\theta^{4}} + \\ + \frac{48\Phi'}{h^{2}}\cot \alpha\left(\frac{1}{2}\frac{\partial u}{\partial x} + \frac{1}{x}\frac{1}{\sin\alpha}\frac{\partial v}{\partial\theta} + \frac{u}{x} + \frac{w}{x}\cot \alpha\right) + \\ - \frac{36p}{h^{3}}\mathrm{tg}\alpha\left(x\frac{\partial w}{\partial x} + \frac{x^{2}}{2}\frac{\partial^{2}w}{\partial x^{2}} + \frac{1}{\sin^{2}\alpha}\frac{\partial^{2}w}{\partial\theta^{2}}\right) = 0. \end{aligned}$$

By using these equations the stability problem has been investigated in [6]. Later equations (4.1) will be used as basic equations for solving step - by -step the stability problem of conical shell made of material with general hardening characteristics.

c) In the case of conical shells made of general hardening material, relation $\sigma_u^0 = \Phi(s)$ represents a material function precisely determined for a given material, it is a curve of the stress intensity σ_u^0 versus the arc - length s of strain trajectory. In general the slope of a tangent to the material curve varies from point to point. Otherwise in the considered conical shell subjected to external pressure

$$\sigma_u^0 = \frac{\sqrt{3} p x}{2 h} t g \alpha = \sigma_M^0 t,$$

i.e. at each point on the generatrix we can know the value of stress intensity σ_u^0 , which is respect to one point on the material curve with coordinates (σ_u^0, s) . Therefore within s and t there is a relation one - to - one, it means that with each value t we can get the value σ_u^0 , then according to the graph $\sigma_u^0 = \Phi(s)$ we determine the value s respectively and then the value $\Phi'(s)$ (the slope of tangent to the material curve) and $N = \sigma_u^0/s$.

Implementing a linearization of the general hardening curve as a piecewise linear hardening one, such that in each interval $[s_i, s_{i+1}]$ we obtain $\Phi'(s) = \Phi'(s_i) = g_i = const$. Consequently at different intervals the quantity $\Phi'(s)$ gets different but constant values, then:

$$\frac{\partial \Phi'}{\partial x} = \frac{\partial^2 \Phi'}{\partial x^2} = 0 \quad \text{or} \quad \frac{\partial \Phi'}{\partial t} = \frac{\partial^2 \Phi'}{\partial t^2} = 0,$$
$$N = g_i + \overline{N} \quad \text{with} \quad \overline{N} = \frac{g_i b_i}{\sigma_u^0 - b_i} = \frac{g_i b_i}{\sigma_M^0 t - b_i},$$

where

$$b_i = \sigma_i - g_i s_i, \quad \sigma_i = \sigma_u^0(s_i), \quad t_i = \frac{\sigma_i}{\sigma_M^0}.$$

With respect to each interval $[s_i, s_{i+1}]$ the stability equations are in the form similar to (4.1).

Taking the solution

$$u = \sum_{n=1}^{N_0} A_n U(t) \cos n\theta, \quad v = \sum_{n=1}^{N_0} B_n V(t) \sin n\theta, \quad w = \sum_{n=1}^{N_0} C_n W(t) \cos n\theta.$$

where A_n, B_n, C_n - constants and substituting them into equations (4.1) we obtain

$$\begin{split} \varPhi(t,\theta) &\equiv \sum_{n=1}^{N_0} \left\{ A_n \Big[(4U''t + 4U') \varPhi' - \big(4 + \frac{n^2}{\sin^2 \alpha}\big) \frac{U}{t} \varPhi' + \right. \\ &+ \overline{N} \big(3U''t + 3U' - \frac{n^2}{\sin^2 \alpha} \frac{U}{t} + 3\frac{\partial \overline{N} U'}{\partial t t} \big) \Big] + \\ &+ B_n \Big[\frac{\varPhi' n}{\sin \alpha} \big(3V' - 5\frac{V}{t} \big) + \overline{N} \big(\frac{n}{\sin \alpha} V' - \frac{n^2}{\sin \alpha} \frac{V}{t} \big) \Big] + \\ &+ C_n 2 \varPhi' \cot g \alpha \big(W' + 2\frac{V}{t} \big) \Big\} \cos n\theta = 0, \end{split}$$

$$\begin{split} \Psi(t,\theta) &\equiv \sum_{n=1}^{N_0} \left\{ A_n \Big[\Phi' \frac{n}{\sin\alpha} \Big(-3U' - 5\frac{U}{t} \Big) + \overline{N} \frac{n}{\sin\alpha} \Big(-U' - \frac{U}{t} \Big) + \frac{n}{\sin\alpha} \frac{U}{t} \frac{\partial \overline{N}}{\partial t} \Big] + \right. \\ &+ B_n \Big[\Phi' \Big(tV'' + V' - \frac{V}{t} \Big) - \frac{4n^2 \Phi'}{\sin^2 \alpha} \frac{V}{t} + \overline{N} \big(tV'' + V' - \frac{V}{t} \big) + \frac{\partial \overline{N}}{\partial t} \big(tV' - V \big) \big] - \\ &- C_n \Phi' \frac{4n}{\sin\alpha} \frac{W}{t} \cot \alpha \Big\} \sin n\theta = 0, \end{split}$$

$$\begin{split} \Omega(t,\theta) &\equiv \sum_{n=1}^{N_0} \left\{ A_n \Big[24\Phi' \cot \alpha \left(\frac{l}{h}\right)^2 (U'+2\frac{U}{t}) \Big] + B_n \Big[48\Phi' \cot \alpha \alpha \frac{n}{\sin \alpha} \left(\frac{l}{h}\right)^2 \frac{V}{t} \Big] \right. \\ &+ C_n \Phi' \Big[4W^{(4)}t + 8W^{(3)} - 4\frac{W''}{t} - 8\frac{n^2}{\sin^2 \alpha} \frac{W''}{t} + 4\frac{W'}{t} + \\ &+ \left(4\frac{n^2}{\sin^2 \alpha} - 16\right) \frac{n^2}{\sin^2 \alpha} \frac{W}{t^3} + 48 \left(\frac{l}{h}\right)^2 \cot 2^2 \alpha \frac{W}{t} \Big] + \\ &+ C_n \overline{N} \Big[3W^{(4)}t + 6W^{(3)} - 4\frac{n^2}{\sin^2 \alpha} \frac{W''}{t} + 4\frac{n^2}{\sin^2 \alpha} \frac{W'}{t} - 4\frac{n^2}{\sin^2 \alpha} \frac{W}{t^3} \Big] + \\ &+ C_n p \, 18tg\alpha \left(\frac{l}{h}\right)^3 \Big[-t^2 W'' - 2tW' + 2\frac{n^2}{\sin^2 \alpha} W \Big] + \\ &+ C_n \Big[6\frac{\partial \overline{N}}{\partial t} W^{(3)}t + 3\frac{\partial^2 \overline{N}}{\partial t^2} tW'' \Big] + \\ &+ C_n \Big[6\frac{\partial \overline{N}}{\partial t} W'' - 4\frac{n^2}{\sin^2 \alpha} \frac{\partial \overline{N}}{\partial t} \frac{W'}{t} + 4\frac{n^2}{\sin^2 \alpha} \frac{W}{t^2} \frac{\partial \overline{N}}{\partial t} \Big] \Big\} \cos n\theta = 0. \end{split}$$

Functions U(t), V(t), W(t) are chosen, such that the boundary conditions (3.2), (3.3) are satisfied.

$$U(t) = t(1-t)^2$$
, $V(t) = t(1-t)$, $W(t) = t^2(1-t)^3$.

Applying the Bubnov - Galerkin's method into equations (4.2), we obtain

$$\sum_{j=0}^{t_{j+1}} \int_{t_j}^{t_{j+1}} dt \int_{0}^{2\pi} \Phi(t,\theta) t (1-t)^2 \cos k\theta \, d\theta = 0,$$

$$\sum_{j=0}^{t_{j+1}} \int_{t_j}^{t_{j+1}} dt \int_{0}^{2\pi} \Psi(t,\theta) t (1-t) \sin k\theta \, d\theta = 0,$$

$$\sum_{j=0}^{t_{j+1}} \int_{t_j}^{t_{j+1}} dt \int_{0}^{2\pi} \Omega(t,\theta) t^2 (1-t)^3 \cos k\theta \, d\theta = 0.$$
 (4.3)

with $k = \overline{0, N_0}$.

Using the orthogonality of trigonometric functions we can see that with respect to each $k \equiv n$ obtaining 3 linearly algebraic homogeneous equations of unknown A_n, B_n, C_n separately; totally we get N_0 sets of 3 separate equations of unknown with $n = 1, 2, \dots N_0$

respectively. Since A_n, B_n, C_n are not simultaneously equal to zero, so the determinant of their coefficients must be vanish, it leads to the relationship for determining the critical load.

About the value $t_s = x_s/l$ which can be defined by using the iterative method, so called elastic solution method. At the first iteration k = 0 suppose the shell in elastic state, the critical load $p_{cr}^{(0)}$ is determined as in [7], then the value $\sigma_M^{(0)}$ is known. If $\sigma_M^{(0)} t < \sigma_s$ for all t, the iterative process can be finished and the instability of shell occurs in elastic range. If $\sigma_M^{(0)} t \ge \sigma_s$, we calculate $t_s^{(0)} = \sigma_s/\sigma_M^{(0)}$ for getting the number of intervals from $t_s^{(0)}$ to 1, on which Φ' takes different constant values and then substitute into (4.3) for the next iteration.

Note that in each iteration one obtains N_0 values of p, the minimum value of which is chosen for the critical p_{cr} . Iterative process can be finished when the difference of results in two consecutive iterations is smaller than a given small value. Using just obtained critical load p_{cr} the elasto - plastic zones boundary can be defined as follows

$$\frac{x_s}{l} = t_s = \frac{2\sigma_s h}{\sqrt{3} \, l \, p_{cr} \, \mathrm{tg} \, \alpha}.$$

5. NUMERICAL EXAMPLE

Consider in turn the stability problem of conical shells made of different materials with:

- Linearly hardening characteristics

$$E = 2, 6.10^{6} (kG/cm^{2}), \quad \sigma_{s} = 4.10^{3} (kG/cm^{2}), \quad g = 0, 43.10^{6} (kG/cm^{2})$$

- General hardening characteristics

$$E = 2, 6.10^6 (kG/cm^2), \quad \sigma_s = 4.10^3 (kG/cm^2);$$

the relation $\sigma_u^0 = \Phi(s)$ is given in the Table 1.

σ_i	4000	4260	4440	4540	4640	4720	4780	4900
$g_i.10^6$	0.853	0.48	0.42	0.32	0.26	0.23	0.22	0.20
σ_i .	5000	5080	5160	5400	5500	5620	5700	5800
$g_i.10^6$	0.16	0.15	0.14	0.12	0.12	0.12	0.11	0.10

Table 1. Relation between stress intensity and hardening modulus

- Elasto - plastic material (almost perfectly plastic material)

$$E = 2.6.10^{6} (kG/cm^{2}), \quad \sigma_{s} = 4.10^{3} (kG/cm^{2}), \quad g = 0.08.10^{6} (kG/cm^{2})$$

Firstly, the calculation is carried out with a conical shell of $\alpha = \pi/4$, the series expansion from n = 1 to n = 15 and 10 iterations. The results of critical loads depending on slenderness l/h of the shell with different materials are given in the Table 2 and illustrated on the Fig. 1.

p_{cr}	l/h								
(kG/cm^2)	500	600	700	800	900	1000	1100	1200	1300
elastic	25.50	17.61	12.93	9.90	7.78	6.32	5.21	4.38	3.72
linear hardening	17.07	13.70	11.05	8.50	7.37	6.32	5.21	4.38	3.72
general hardening	13.00	10.87	8.96	7.90	7.03	6.32	5.21	4.38	3.72
perfectly plastic	10.00	8.59	7.39	6.48	5.77	5.30	5.21	4.38	3.72

Table 2. Relation between critical loads and slenderness with various materials

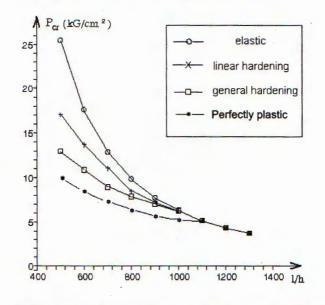


Fig.1. Effect of hardening characteristics on critical loads

Secondly, analysis is fulfilled with a conical shell made of general hardening material, the open angle at the shell top varies from $\pi/4$ to $\pi/2$, while the slenderness l/h = 400. Obtained results are presented in the Table 3.

Table 3. Relation between critical loads, elastic- plastic boundary and open angle

α	0.78	0.80	0.82	0.84	0.88	0.90
$p_{cr}(\mathrm{kG/cm^2})$	16.45	14.5	13.0	6.4	2.7	0.9
$t_s = x_s/l$	0.7	0.75	0.79	1.0	1.0	1.0

5. DISCUSSION

From the results we can see that

- For conical shells when the ratio l/h increases, then the critical load decreases while the value x_s/l increases, i.e. the plastic zone is reduced. The ratio l/h is reaching some value the loss of stability of the shell will occur in elastic range (in considered example with the shell of $\alpha = \pi/4$ and $l/h \ge 1000$). - With materials of the same E, σ_s and the shell of the same ratio l/h and open angle α the critical loads decrease in turn elastic, linear hardening, general hardening and perfectly plastic material. The greater hardening modulus $\Phi'(s)$ of material results in greater critical load.

- With the same material and the same ratio l/h the critical loads decrease when the open angle α at shell top increases, i.e for a flatter conical shell the critical load is smaller.

In conclusion, an analysis procedure has been developed for solving the elastoplastic stability problem of conical shells.

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ẢNH HƯỞNG CỦA ĐẶC TRƯNG TÁI BỀN CỦA VẬT LIỆU ĐẾN TẢI TRỌNG TỚI HẠN TRONG BÀI TOÁN ỔN ĐỊNH ĐÀN-ĐẢO CỦA VỎ NÓN

Trong bài báo này đã thiết lập các phương trình cơ bản của bài toán ổn định ngoài giới hạn đàn hồi của vỏ nón dựa trên lý thuyết quá trình đàn dẻo và tiêu chuẩn tồn tại các dạng cân bằng lân cận, khi trạng thái màng trước khi mất ổn định là không thuần nhất. Lời giải của bài toán nhận được nhờ áp dụng phương pháp Bubnov-Galerkin và phương pháp tham số tải, qua đó khảo sát ảnh hưởng của đặc trưng tái bền của vật liệu đến tải trọng tới hạn của vỏ nón chịu tác dụng của áp suất ngoài.