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Dedicated to Professor J.N. Reddy on the Occasion of His 75th Birthday

A MODIFIED AVERAGING OPERATOR WITH SOME APPLICATIONS

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Abstract. The paper presents a new approach to the conventional averaging in which the role of boundary values is considered in a more detailed way. It results in a new weighted local averaging operator (WLAO) taking into account the particular role of boundary values. A remarkable feature of WLAO is that this operator contains a parameter of boundary regulation p and depends on a local value h of the integration domain. By varying these two parameters one can regulate the obtained approximate solutions in order to get more accurate ones. It has been shown that the combination of WLAO with Galerkin method can lead to an effective approximate tool for the buckling problem of columns and for the frequency analysis of free vibration of strongly nonlinear systems.

Keywords: weighted local averaging, Galerkin method, buckling, Euler column, free vibration, strong nonlinearity.

1. INTRODUCTION

In mathematics, an operator is generally a mapping or function that operates on elements of space to create elements of another space. One of the most natural and popular models of operators is integral operators that are widely and and effectively used in many fields of science and engineering in general, and in applied mathematics and mechanics, in particular. Indeed it is impossible to review the applications of integral operators in all branches of science and engineering. Well-known examples of integral operator are integral transforms such as Laplace and Fourier transforms, which are mappings between two function spaces. Integral operator is used to present functionals that are the objects of the study of the variational theory and functional analysis Oden and Reddy [1], Reddy [2]. Integral operator is an effective tool for presenting energies of a body in a deformed state, such as strain, stress energies; kinetic and potential energies for deformable body Reddy [3]. The use of integral operators allows to introduce several integral principles of mechanics, for example, variational principles that support the formulation of equations of motion or/and relationships between stresses, strains or deformations, displacements Reddy [4,5]. Action integral is used to formulate Hamilton's principle, which allows the derivation of differential equations of motion of mechanical systems composed of rigid and/or deformable bodies Reddy [3].

Among integral operators averaging operators form an important class because they can combine all values of a function into an average value. For example, for one-dimensional structures, the conventional averaging operator is used as a integration over total structural length. For this reason, the conventional averaging (CA) can often be cited as simple or arithmetic averaging based on the suggestion that all values have the same role for the function in question. Along with the conventional averaging, the weighted averaging (WA) is another interesting and effective approach based on the philosophy that each value has a different contribution to the function. For example weighted integrals are used in weighted-residual methods to extract algebraic equations from the governing differential equation in weak form for a finite model for structures Oden and Reddy [6], Reddy [7]. Both-conventional and weighted averaging-are widely used in practice but each type is more appropriate to use than the other for certain purposes and applications.

The aim of this paper is to present a simple form of weighted averaging operator taking into account the particular role of boundary values of a function. It is obtained that the connection of WA with Galerkin method can lead to an effective approximate tool for the buckling problem of columns and the frequency analysis of free vibration of nonlinear systems. With respect to analytical investigations, many attempts have been given to improve the accuracy of the first order approximate solution of Galerkin method, since this solution can usually be obtained in a simple form. Exact solutions for buckling of structural members including various cases of columns, beams, arches, rings, plates, and shells with variable cross-section, variable axial forces and different boundary conditions were given in the book by Wang C.M., Wang C.Y. and Reddy [8]. The exact solutions for buckling problem are used in this paper to check the accuracy of Galerkin method with weighted averaging. An interesting discussion about the implementation of Galerkin method for stepped beams is given recently by Elishakoff, Ankitha and Marzani [9]. The paper is organized as follows: a weighted local averaging (WLA) is presented in the second section. The application of WLA for the buckling problem of columns and the frequency analysis of free vibration of nonlinear systems is presented in Sections 3 and 4, respectively. Conclusions and further investigations are summarized in Section 5.

2. WEIGHTED LOCAL AVERAGING WITH EMPHASIS ON BOUNDARY DOMAINS

Let g(x) is an integrable deterministic function of $x \in [0,1]$ and h is a local value in [0,1]. The average of g(x) over the interval [0,1] is given by an integral as follows

$$\langle g(x)\rangle = \int_{0}^{1} g(x)dx, \qquad (1)$$

where $\langle . \rangle$ denotes the conventional averaging operator. The average (1) is called simple average or arithmetic mean because all values of g(x) are treated equally and assigned equal weight. However, values of g(x) may be weighted for the reason that they belong to different domains of the interval [0, 1]. It is well known that the boundary conditions play a key role in Mechanics of solids and structures Fenner and Reddy [10]. To develop this point of view, in addition to the conventional arithmetic average, we consider the following integral taken over the global domain and over some local boundary domains as follows

$$\left\langle g(x),h\right\rangle_{p} = q \int_{0}^{1} g(x) \mathrm{d}x + p \left(\int_{0}^{h} g(x) \mathrm{d}x + \int_{1-h}^{1} g(x) \mathrm{d}x \right), \tag{2}$$

where the left side is a new notation denoting the weighted local averaging (WLA) of g(x) at a local value h, q and p are weights, second term involves values of g(x) integrated in the boundary domains [0,h] and [1-h,1]. It is noted that the parameter p is introduced in Eq. (2) to highlight a special role of g(x) in boundary domains and hence p can be quoted as parameter of boundary regulation. We will require that the operator (2) is conservative so one has the following equality

$$\langle 1,h\rangle_p = 1. \tag{3}$$

Using Eq. (2) one obtains from Eq. (3)

$$q\int_{0}^{1} 1dx + p\left(\int_{0}^{h} 1dx + \int_{1-h}^{1} 1dx\right) = q + 2ph = 1,$$
(4)

$$q = 1 - 2ph. \tag{5}$$

Substituting Eq. (5) into Eq. (2) gives

$$\langle g(x),h\rangle_p = (1-2ph)\int_0^1 g(x)dx + p\left(\int_0^h g(x)dx + \int_{1-h}^1 g(x)dx\right).$$
 (6)

Hence, values of the function g(x) are weighted for the reason that the values in the boundary regions are calculated once more and then multiple with a weight p. The weighted local averaging is coincident with the conventional averaging for p = 0.

It is worth to note that WLA of g(x) defined by Eq. (6) is a function of *h*. Some following properties are obtained for WLA (6):

Property. The weighted local average of g(x) is equal to conventional average of g(x) at three local values h = 0; 0.5 and 1, i.e. one has

$$\langle g(x), 0 \rangle_{p} = (1-0) \int_{0}^{1} g(x) dx + p \left(\int_{0}^{0} g(x) dx + \int_{1}^{1} g(x) dx \right) = \int_{0}^{1} g(x) dx = \langle g(x) \rangle,$$

$$\langle g(x), 0.5 \rangle_{p} = (1-p) \int_{0}^{1} g(x) dx + p \left(\int_{0}^{0.5} g(x) dx + \int_{0.5}^{1} g(x) dx \right) = \int_{0}^{1} g(x) dx = \langle g(x) \rangle,$$

$$\langle g(x), 1 \rangle_{p} = (1-2p) \int_{0}^{1} g(x) dx + p \left(\int_{0}^{1} g(x) dx + \int_{0}^{1} g(x) dx \right) = \int_{0}^{1} g(x) dx = \langle g(x) \rangle.$$

$$(7)$$

For one term polynomial x^n one gets

$$\langle x^{n}, h \rangle_{p} = (1 - 2ph) \int_{0}^{1} x^{n} dx + p \left(\int_{0}^{h} x^{n} dx + \int_{1-h}^{1} x^{n} dx \right)$$

$$= (1 - 2ph) \frac{1}{n+1} + p \left(\frac{h^{n+1}}{n+1} + \frac{1 - (1 - h)^{n+1}}{n+1} \right)$$

$$= \frac{1}{n+1} \left(1 + p \left(1 - 2h + h^{n+1} - (1 - h)^{n+1} \right) \right).$$
(8)

In particular one has

$$\langle x,h \rangle_{p} = \frac{1}{2}, \left\langle x^{2},h \right\rangle_{p} = \frac{1}{3} \left(1 + ph(1-h)(1-2h) \right), \left\langle x^{3},h \right\rangle_{p} = \frac{1}{4} \left(1 + 2ph(1-h)(1-2h) \right).$$
(9)

For harmonic functions one gets

$$\langle \cos(2\pi nx), h \rangle_p = (1 - 2ph) \int_0^1 \cos(2\pi nx) dx + p \left(\int_0^h \cos(2\pi nx) dx + \int_{1-h}^1 \cos(2\pi nx) dx \right)$$

$$= \frac{p}{\pi n} \sin 2\pi nh,$$

$$\langle \sin(2\pi nx), h \rangle_p = (1 - 2ph) \int_0^1 \sin(2\pi nx) dx + p \left(\int_0^h \sin(2\pi nx) dx + \int_{1-h}^1 \sin(2\pi nx) dx \right) = 0.$$

$$(10)$$

Some graphics of $\langle x^n, h \rangle_p$ as functions of *h*, or of *p*, or of *h* and *p* both are shown in Figs. 1–2.

1

If function g(x) is expanded into Taylor series

$$g(x) = \sum_{i=0}^{\infty} g_i x^i,\tag{11}$$



Fig. 1. WLA of x^n as a function of (a) *h* only and for n = 1, 2, 3; p = 0.5; (b) *h* only and for n = 2; p = 0, 0.5, 1; (c) *p* only and for n = 1, 2, 3; h = 0.25



Fig. 2. WLA value of x^n as a function of *h* and *p* with (a) n = 1; (b) n = 2 and (c) n = 3

the weighted local average of g(x) can be formulated in its explicit form using the linearity of the weighted averaging operator and Eq. (8)

$$\left\langle g(x),h\right\rangle_{p} = \left\langle \sum_{i=0}^{\infty} g_{i}x^{i},h\right\rangle_{p} = \sum_{i=0}^{\infty} g_{i}\left\langle x^{i},h\right\rangle_{p} = \sum_{i=0}^{\infty} \frac{g_{i}}{i+1}(1+p(1-2h+h^{i+1}-(1-h)^{i+1})).$$
(12)

The determination of weights for a data given is a difficult and complicated problem which needs an adequate strategy. So far there is no general theory about this. In this paper, we consider three weight values that are p = 0.25, 0.5 and 1. To clarify the meaning of the parameter of boundary regulation one represents Eq. (6) in the following form

$$\langle g(x),h\rangle_{p} = (1-2ph)\int_{0}^{1} g(x)dx + p\left(\int_{0}^{h} g(x)dx + \int_{1-h}^{1} g(x)dx\right)$$

$$= \int_{0}^{1} g(x)dx + p\left(\int_{0}^{h} g(x)dx + \int_{1-h}^{1} g(x)dx - 2h\int_{0}^{1} g(x)dx\right).$$
(13)

The expression

$$B(g(x),h) = \left(\int_{0}^{h} g(x)dx + \int_{1-h}^{1} g(x)dx - 2h\int_{0}^{1} g(x)dx\right),$$
(14)

can be interpreted as a measure charactering the effect of boundary values of the function g(x). It is seen from Eq. (13) when p = 0.5 the influence level of the effect of boundary values is equal to a half of the one of the averaged value of the function g(x), and when p = 1 two influence levels are considered

the same. A detailed investigation of B(g(x), h) might be an interesting topic of coming research. In the next sections two applications of WLA will be illustrated for the buckling problem of Euler columns and the frequency analysis of free vibration of strongly nonlinear systems.

3. APPLICATION OF WLA TO THE BUCKLING PROBLEM OF EULER COLUMNS

For the elastic buckling of Euler columns [11] consider an elastic column of length *L* subjected to an axial compressive force \bar{P} . The column undergoes a lateral deflection denoted by $\bar{W}(z)$, where *z* denotes the vertical axis coordinate of the column. Using the theory of Euler–Bernoulli beam the lateral deflection of the column is described by the following differential equation

$$\frac{d^2}{dz^2} \left(EI(z) \frac{d^2 \bar{W}(z)}{dz^2} \right) + \bar{P} \frac{d^2 \bar{W}(z)}{dz^2} = 0,$$
(15)

where is the Young's modulus of elasticity, I(z) is the moment of inertia of the column. Let $I(z) = I_0 b(z)$, where I_0 is the moment of inertia of the column at z = 0 and b(z) is a function defined in the interval [0, L] and $b(z) > 0, \forall z \in [0, L]$. By using transformation

$$x = z/L$$
, $W(x) = \overline{W}(z)/L$, $P = \overline{P}L^2/EI_0$,

one gets from Eq. (15)

$$\frac{d^2}{dx^2} \left(b(x) \frac{d^2 W(x)}{dx^2} \right) + P \frac{d^2 W(x)}{dx^2} = 0.$$
 (16)

To solve Eq. (16) one needs to add boundary conditions for column at two points x = 0 and x = 1. In this paper we consider 4 typical types of boundary conditions as given in Tab. 1. For each type of boundary conditions there exists a corresponding comparison function W(x) of polynomial form, $W(x) = \sum_{i=0}^{n=4} C_i x^i$, where C_i are obtained from the boundary conditions. Tab. 1 shows 4 types of boundary conditions and corresponding comparison functions.

Type of boundary conditions	Boundary conditions	Comparison function $W(x)$
Pinned-Pinned Column (P-P)	$W(0) = 0; \frac{d^2 W(0)}{dx^2} = 0$ $W(1) = 0; \frac{d^2 W(1)}{dx^2} = 0$	$x^4 - 2x^3 + x$
Clamped-Pinned Column (C-P)	$W(0) = 0; \frac{dW(0)}{dx} = 0$ $W(1) = 0; \frac{d^2W(1)}{dx^2} = 0$	$2x^4 - 5x^3 + 3x^2$
Clamped-Sliding Column (C-S)	$W(0) = 0; \frac{dW(0)}{dx} = 0$ $\frac{dW(1)}{dx} = 0; \frac{d^3W(1)}{dx^3} + P\frac{dW(1)}{dx} = 0$	$x^4 - 4x^3 + 4x^2$
Clamped-Clamped Column (C-C)	$W(0) = 0; \frac{dW(0)}{dx} = 0$ $W(1) = 0; \frac{dW(1)}{dx} = 0$	$x^4 - 2x^3 + x^2$

 Table 1. Different types of columns with corresponding boundary conditions and comparison functions

The first order approximate solution of the buckling problem described by Eq. (16) can be obtained by using Galerkin method for one term comparison function as follows

$$\left\langle \left(\frac{\mathrm{d}^2}{\mathrm{d}x^2} \left(b(x)\frac{\mathrm{d}^2 W(x)}{\mathrm{d}x^2}\right) + P\frac{\mathrm{d}^2 W(x)}{\mathrm{d}x^2}\right) W(x) \right\rangle = 0.$$
(17)

Eq. (17) leads to the approximate buckling load obtained by Galerkin method with CA

$$P_{ca} = -\frac{\left\langle \frac{\mathrm{d}^2}{\mathrm{d}x^2} \left(b(x) \frac{\mathrm{d}^2 W(x)}{\mathrm{d}x^2} \right) W(x) \right\rangle}{\left\langle \frac{\mathrm{d}^2 W(x)}{\mathrm{d}x^2} W(x) \right\rangle}.$$
(18)

If in Eq. (18) one replaces the conventional averaging by the weighted local averaging and gets

$$\left\langle \left(\frac{\mathrm{d}^2}{\mathrm{d}x^2} \left(b(x)\frac{\mathrm{d}^2 W(x)}{\mathrm{d}x^2}\right) + P\frac{\mathrm{d}^2 W(x)}{\mathrm{d}x^2}\right) W(x), h \right\rangle_p = 0.$$
(19)

Then one has the approximate buckling load obtained by Galerkin method with WLA

$$P(p,h) = -\frac{\left\langle \frac{\mathrm{d}^2}{\mathrm{d}x^2} \left(b(x) \frac{\mathrm{d}^2 W(x)}{\mathrm{d}x^2} \right) W(x), h \right\rangle_p}{\left\langle \frac{\mathrm{d}^2 W(x)}{\mathrm{d}x^2} W(x), h \right\rangle_p}.$$
(20)

Or in the explicit form

P(p,h) =

$$-\frac{(1-2ph)\int_{0}^{1}\frac{d^{2}}{dx^{2}}\left(b(x)\frac{d^{2}W(x)}{dx^{2}}\right)W(x)dx+p\left(\int_{0}^{h}\frac{d^{2}}{dx^{2}}\left(b(x)\frac{d^{2}W(x)}{dx^{2}}\right)W(x)dx+\int_{1-h}^{1}\frac{d^{2}}{dx^{2}}\left(b(x)\frac{d^{2}W(x)}{dx^{2}}\right)W(x)dx\right)}{(1-2ph)\int_{0}^{1}\frac{d^{2}W(x)}{dx^{2}}W(x)dx+p\left(\int_{0}^{h}\frac{d^{2}W(x)}{dx^{2}}W(x)dx+\int_{1-h}^{1}\frac{d^{2}W(x)}{dx^{2}}W(x)dx\right)}$$
(21)

It is seen from Eq. (21) that the critical load determined by Galerkin method with WLA is a function of parameter p and local value h. If the value p is given, the buckling load will be chosen as the lowest value of P(p,h) in the interval [0.1] i.e.

$$P_{wla}(p) = \min_{h \in [0,1]} P(p,h).$$
(22)

It is clearly seen from Eqs. (18) and (21) that the approximate buckling load obtained by Galerkin method with CA is corresponding to the case p = 0 of the approximate buckling load obtained by Galerkin method with WLA.

3.1. Columns with constant cross-section

Tab. 2 shows the buckling loads obtained by Galerkin method with CA (p = 0) and WLA (p = 0.25 and p = 0.5) for all 4 types of boundary conditions and compares those with exact values from [8].

 Table 2. Accuracy of approximate buckling loads for different types of column with constant cross-section

Type of BC	P _{exact} [8]	$P_{ca} = P_{wla}, p = 0$	%E	$P_{wla}, p = 0.25$	%E	$P_{wla}, p = 0.5$	%E
P-P	9.8696	9.8823	0.129	9.6474	2.252	9.443	4.326
C-P	20.1907	21.0000	4.008	20.3582	0.833	19.8113	1.876
C-S	9.8696	10.5000	6.387	9.9931	1.252	9.5289	3.452
C-C	39.4784	42.0000	6.387	38.9438	1.358	36.591	7.319

It is seen from Tab. 2 that percent errors of solutions obtained by WLA for p = 0.25 are much smaller than the ones of solutions obtained by of CA for 3 types of columns (C-P, C-S, C-C), for example, for C-P type two errors are 4.008% and 0.833%, respectively Besides, the accuracy of WLA for p = 0.25 is much better in average compared to the accuracy of CA. The percent errors of WLA for p = 0.5 are smaller than the ones of CA for 2 types of columns (C-P and C-S) and larger than the ones of CA for 2

remaining types of columns (P-P and C-C). The accuracy of WLA for p = 0.5 and of CA is nearly the same in average.

3.2. Columns with variable cross-section given by exponential function

In this section, we consider columns with variable cross-section whose moment of inertia is given by exponential function

$$EI(x) = EI_0 \exp(-BLx).$$
⁽²³⁾

Tabs. 3–6 show the critical buckling loads obtained by WLA for p = 0.25 and p = 0.5 as well as the ones of CA for 4 types of columns with variable cross-section. It is seen from all tables that the accuracy of solutions obtained by WLA is significantly improved in comparison to the one of solutions obtained by of CA, specially, when the change of the cross-section is increased. For example, in Tab. 3 starting from BL = 1, 2 the percent errors of WLA for p = 0.25 are 0.728% and 10.079% while the ones of CA are 4.714% and 19.560%, respectively. The accuracy of solutions obtained by WLA for p = 0.5 is much improved, namely for BL = 1.5 and 2 the percent errors of WLA are 0.806% and 1.821% while the ones of solutions obtained by CA are 10.698% and 19.560%, respectively. In Tabs. 4–6 the accuracy of solutions obtained by WLA is always much better than the one of solutions obtained by WLA for p = 0.5 is much better than the accuracy of solutions obtained by WLA for p = 0.5 is much better than the accuracy of solutions obtained by CA when BL > 0. For columns with variable cross-section the accuracy of solutions obtained by WLA for p = 0.5 is much better than the accuracy of solutions obtained by WLA for p = 0.5 is much

Table 3. Accuracy of approximate buckling loads for Pinned-Pinned column with exponential moment of inertia

BL	Pexact [8]	$P_{ca} = P_{wla}, p = 0$	%E	$P_{wla}, p = 0.25$	%E	$P_{wla}, p = 0.5$	%E
0	9.8696	9.8823	0.129	9.6474	2.252	9.4427	4.326
0.5	7.6340	7.7308	1.268	7.5192	1.504	7.3348	3.920
1	5.8270	6.1017	4.714	5.869	0.728	5.6671	2.745
1.5	4.3890	4.8585	10.698	4.589	4.551	4.3536	0.806
2	3.2640	3.9024	19.560	3.593	10.079	3.3234	1.821

BL	P _{exact} [8]	$P_{ca}=P_{wla}, p=0$	%E	$P_{wla}, p = 0.25$	%E	$P_{wla}, p = 0.5$	%Е
0	20.1907	21.0000	4.008	20.3582	0.833	19.8113	1.876
0.5	15.6400	17.2408	10.236	16.8155	7.516	16.4524	5.194
1	11.9900	14.4716	20.698	14.1140	17.714	13.8081	15.164
1.5	9.0980	12.4117	36.422	12.0412	32.350	11.7241	28.865
2	6.8390	10.8627	58.835	10.4401	52.656	10.0786	47.369

Table 4. Accuracy of approximate buckling loads for Clamped-Pinned column with exponential moment of inertia

Table 5. Accuracy of approximate buckling loads for Clamped-Sliding column with exponential moment of inertia

BL	P _{exact} [8]	$P_{ca}=P_{wla}, p=0$	%E	$P_{wla}, p = 0.25$	%E	$P_{wla}, p = 0.5$	%E
0	9.8696	10.5000	6.387	9.9931	1.252	9.5289	3.452
0.5	7.6830	9.8408	28.086	9.1585	19.205	8.5336	11.071
1	5.9730	8.8815	48.694	8.1329	36.161	7.4473	24.682
1.5	4.6330	7.9233	71.019	7.1769	54.907	6.4931	40.150
2	3.5800	7.0808	97.788	6.3760	78.100	5.7304	60.066

Table 6. Accuracy of approximate buckling loads for Clamped-Clamped column with exponential moment of inertia

BL	P _{exact} [8]	$P_{ca}=P_{wla}, p=0$	%E	$P_{wla}, p = 0.25$	%E	$P_{wla}, p = 0.5$	%E
0	39.4784	42.0000	6.387	38.9438	1.358	36.5906	7.319
0.5	30.6000	33.2473	8.651	30.7427	0.4663	28.8142	5.836
1	23.4900	27.1709	15.670	24.7742	6.108	23.1952	1.255
1.5	17.8600	22.8761	28.086	20.3307	16.072	19.0786	6.823
2	13.4600	19.7815	46.965	17.1182	31.183	16.0222	19.036

4. APPLICATION OF WLA TO THE FREQUENCY ANALYSIS OF STRONGLY NONLINEAR SYSTEMS

In this section the second application of the weighted local averaging is one to the analysis of free vibration of strongly nonlinear systems which have attracted an increasing interest of scientists and engineers in the field of dynamic nonlinear problems [12–19]. For strongly nonlinear systems there exists no small parameter in the governing equation so the traditional perturbation methods cannot be applied directly. Many techniques have appeared in the literature for strongly nonlinear systems, for example, the weighted linearization method [12], the modified Lindstedt-Poincare method [13], the perturbation incremental method [14], equivalent non-linearization method [15] and the homotopy perturbation method [16].

We consider the free vibration of a strongly nonlinear system described by the following differential equation

$$\ddot{z}(t) + f(z(t)) = 0, \quad z(0) = A, \quad \dot{z}(0) = 0,$$
(24)

where dot denotes the differentiation with respect to t, f(z) is a nonlinear function of z, A is a constant value. We will investigate the frequency dependence on the initial amplitude A of a periodic solution of Eq. (24) by Galerkin method with WLA. To do that we seek the approximate periodic solution of Eq. (24) in the form

$$z(t) = A\cos\omega t = \cos 2\pi x,\tag{25}$$

where ω is unknown frequency and x is a new variable which is related to the variable t as follows

$$x = \frac{\omega}{2\pi}t.$$
 (26)

It is seen from Eq.
$$(26)$$
 that in a period of vibration, T , one has

$$0 \le t \le T = \frac{2\pi}{\omega} \text{ then } 0 \le x \le 1.$$
(27)

Substituting Eq. (25) into Eq. (24) one gets the residual of equation

$$R(t) = \ddot{z}(t) + f(z(t)) = -\omega^2 A \cos \omega t + f(A \cos \omega t),$$
(28)

or

$$R(x) = -\omega^2 A \cos \omega t + f(A \cos \omega t) = -\omega^2 A \cos 2\pi x + f(A \cos 2\pi x).$$
⁽²⁹⁾

Using Galerkin method with CA one gets the condition of orthogonality in the form

$$\frac{\omega}{2\pi} \int_{0}^{2\pi/\omega} R(t)A\cos\omega t dt = \int_{0}^{1} R(x)A\cos 2\pi x dx$$

$$= \int_{0}^{1} \left(-\omega^{2}A\cos 2\pi x + f(A\cos 2\pi x)\right)A\cos 2\pi x dx = 0.$$
(30)

Hence the approximate frequency ω_{ca} obtained by Galerkin method with CA takes the expression

$$\omega_{ca}^{2} = \frac{\int_{0}^{1} f(A\cos 2\pi x)\cos 2\pi x dx}{A\int_{0}^{1} \cos^{2} 2\pi x dx}.$$
(31)

Using Galerkin method with WLA one writes the condition of orthogonality in the form

$$\left\langle \left(-\omega^2 A \cos 2\pi x + f(A \cos 2\pi x)\right) A \cos 2\pi x, h\right\rangle_p = 0, \tag{32}$$

and the approximate frequency ω obtained by Galerkin method with WLA takes the expression $\omega^2 -$

$$\frac{(1-2ph)\int_{0}^{1}f(A\cos 2\pi x)\cos 2\pi x dx + p\left(\int_{0}^{h}f(A\cos 2\pi x)\cos 2\pi x dx + \int_{1-h}^{1}f(A\cos 2\pi x)\cos 2\pi x dx\right)}{(1-2ph)\int_{0}^{1}A\cos^{2}2\pi x dx + p\left(\int_{0}^{h}A\cos^{2}2\pi x dx + \int_{1-h}^{1}A\cos^{2}2\pi x dx\right)}.$$
(33)

It is seen from Eq. (33) that the frequency determined by Galerkin method with WLA is a function of parameter *p* and local value *h*. When the value *p* is given, the frequency interested, ω_{wla} , will be chosen as the lowest value of $\omega(p, h)$ in the interval (0.1) i.e.

$$\omega_{wla}(p) = \min_{h \in [0,1]} \omega(p,h). \tag{34}$$

It is seen that the approximate frequency ω_{ca} obtained by CA (Eq. (31)) can be obtained from the approximate frequency ω_{wla} obtained by WLA (Eq. (33)) by putting p = 0.

In the next subsections some typical strongly nonlinear systems are considered and the accuracy of approximate frequencies obtained by WLA and CA are evaluated, respectively.

4.1. Generalized Duffing system with strong nonlinearity

A generalized Duffing system with strong nonlinearity has the form

$$\ddot{z}(t) + z^{2n+1}(t) = 0, \quad z(0) = A, \quad \dot{z}(0) = 0,$$
(35)

which is obtained from Eq. (24) when the nonlinear function f(z) takes the form

$$f(z) = z^{2n+1},$$
 (36)

where *n* is a positive, integer number. Substituting Eq. (36) into Eq. (33) leads to the following approximate frequency for the generalized Duffing system (35)

$$\omega_{wla}^{2} = \frac{(1-2ph)\int_{0}^{1} A^{2n} \cos^{2n+2} 2\pi x dx + p\left(\int_{0}^{h} A^{2n} \cos^{2n+2} 2\pi x dx + \int_{1-h}^{1} A^{2n} \cos^{2n+2} 2\pi x dx\right)}{(1-2ph)\int_{0}^{1} \cos^{2} 2\pi x dx + p\left(\int_{0}^{h} \cos^{2} 2\pi x dx + \int_{1-h}^{1} \cos^{2} 2\pi x dx\right)}.$$
 (37)

Zhang [20] obtains the following approximate natural frequency for Eq. (35) using He's energy balance method.

$$\omega_{EBM} = \sqrt{\frac{3A^{2n}}{n+1}} \left(1 - \frac{(2n+2)!!}{(2n+3)!!} \right).$$
(38)

The exact frequency is determined by the following formula [20]

$$\omega_E = \frac{2\pi}{4\int\limits_0^{\pi/2} \frac{\mathrm{d}\theta}{\sqrt{Q}}}, \quad Q = \sqrt{\frac{1}{n+1}} A^{2n} \left(1 + \sin^2\theta + \sin^4\theta + \ldots + \sin^{2n}\theta\right). \tag{39}$$

Tab. 7 shows the approximate frequencies obtained by He's energy balance method (Eq. (38)) and Galerkin method with CA and WLA (Eq. (37)) for different values of integer *n* and compares those with exact frequency (Eq. (39)).

It is seen from Tab. 7 that the accuracy of solutions obtained by WLA for all values p = 0.25; 0.5; 1 is always better than the accuracy of solutions obtained by CA, specially, when *n* is increasing. For example, for n = 1; 2 the percent errors of WLA for p = 0.25; 0.5; 1 are 1.313%, 4.324%; 0.311%, 2.653%; 1.958%, 1.168% while the ones of CA are 2.224%, 5.864%, respectively; for n = 4, 5 the percent errors of

п	ω_E/A^n [20]	$\omega_{ca}/A^{n} = \omega_{wla}/A^{n},$ $p = 0$	E%	ω_{wla}/A^n , p=0.25	E%	ω_{wla}/A^n , p=0.5	E%	ω_{wla}/A^n , p=1	E%	ω_{EBM}/A^n , [20]	E%
1	0.8472	0.8660	2.224	0.8583	1.313	0.8498	0.311	0.8306	1.958	0.8367	1.25
2	0.7468	0.7906	5.864	0.7791	4.324	0.7667	2.653	0.7381	1.168	0.7368	1.34
3	0.6750	0.7395	9.559	0.7259	7.543	0.7111	5.347	0.6771	0.308	0.6673	1.14
4	0.6204	0.7016	13.082	0.6865	10.665	0.6702	8.026	0.6326	1.972	0.6151	0.85
5	0.5772	0.6717	16.376	0.6557	13.615	0.6383	10.594	0.5983	3.660	0.5740	0.54

Table 7. Comparison of approximate frequencies with exact frequency

WLA for p = 0.25; 0.5; 1 are 10.665%, 13.615%; 8.026%, 10.594%; 1.972%, 3.660% while the ones of CA are 13.082%, 16.376%, respectively. The percent errors of WLA for p = 1 are smaller than the ones of He's energy balance method for n = 2 and 3; while they are bigger for n = 1, 4 and 5.

4.2. Nonlinear system with discontinuity term

The second illustration example is the nonlinear oscillator with discontinuity term for which the elastic restoring force is an antisymmetric constant force

$$f(z) = \operatorname{sgn}(z),\tag{40}$$

where the function sgn(z) is defined by

$$\operatorname{sgn}(z) = \begin{cases} -1, & z < 0\\ 0, & z = 0\\ 1, & z > 0 \end{cases}$$
(41)

Substituting Eq. (40) into Eq. (24) leads to the following equation

$$\ddot{z}(t) + \operatorname{sgn}(z(t)) = 0, \quad z(0) = A, \quad \dot{z}(0) = 0.$$
 (42)

This type of nonlinear system has been analyzed by many authors using different analytical approximate methods [21–24]. The exact frequency of the nonlinear system (42) is given in [25]

$$\omega_E = \pi / (2\sqrt{2}A) = 1.110721 / \sqrt{A}. \tag{43}$$

Substituting Eq. (40) into Eq. (33) leads to the following approximate frequency for the nonlinear system with discontinuity term (42)

$$\omega_{wla}^{2} = \frac{(1-2ph)\int_{0}^{1}A\operatorname{sgn}(\cos 2\pi x)\cos 2\pi x dx + p\left(\int_{0}^{h}A\operatorname{sgn}(\cos 2\pi x)\cos 2\pi x dx + \int_{1-h}^{1}A\operatorname{sgn}(\cos 2\pi x)\cos 2\pi x dx\right)}{(1-2ph)\int_{0}^{1}A^{2}\cos^{2}2\pi x dx + p\left(\int_{0}^{h}A^{2}\cos^{2}2\pi x dx + \int_{1-h}^{1}A^{2}\cos^{2}2\pi x dx\right)}$$
(44)

Tab. 8 shows the approximate frequency obtained by He's energy balance method [20] and Galerkin method with CA and WLA (Eq. (44)) for different values of p and compares those with exact frequency [25].

Table 8. Comparison of approximate frequencies with exact frequency

$\sqrt{A}\omega_E$ [25]	$ \begin{array}{l} \sqrt{A}\omega_{ca} = \\ \sqrt{A}\omega_{wla}, \\ p = 0 \end{array} $	E%	$\sqrt{A}\omega_{wla},$ $p = 0.25$	E%	$\sqrt{A}\omega_{wla},\\p=0.5$	E%	$\sqrt{A}\omega_{wla},$ $p=1$	Е%	$\sqrt{A}\omega_{EBM}$ [20]	E%
1.1107	1.1284	1.594	1.1205	0.882	1.1132	0.225	1.0998	0.981	1.1347	2.161

It is seen from Tab. 8 that the accuracy of solutions obtained by WLA for all values p = 0.25; 0.5; 1is better than the accuracies of solutions obtained by CA and by He's energy balance method; and the accuracy of WLA for p = 0.5 is the best.

4.3. System with rational elastic restoring term

Consider the strongly nonlinear oscillator of the form

$$\ddot{z}(t) + 1/z(t) = 0, \quad z(0) = A, \quad \dot{z}(0) = 0.$$
 (45)

In this case the nonlinear function is of follows

$$f(z) = 1/z. \tag{46}$$

The approximate frequency obtained by He [16] using the homotopy perturbation method has the expression

$$\omega_{hnm} = 1.1547 / A. \tag{47}$$

The exact frequency of the system (45) is given in [25]

$$\omega_E = \frac{\sqrt{2\pi}}{2A} = 1.2533/A.$$
 (48)

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Substituting Eq. (46) into Eq. (33) leads to the following approximate frequency for the system (45)

$$\omega_{wla}^{2} = \frac{(1-2ph)\int_{0}^{1} 1+p\left(\int_{0}^{h} 1dx+\int_{1-h}^{1} 1dx\right)}{(1-2ph)\int_{0}^{1} A^{2}\cos^{2}2\pi xdx+p\left(\int_{0}^{h} A^{2}\cos^{2}2\pi xdx+\int_{1-h}^{1} A^{2}\cos^{2}2\pi xdx\right)}$$

$$= \frac{1}{A^{2}\left[(1-2ph)\int_{0}^{1} \cos^{2}2\pi xdx+p\left(\int_{0}^{h} \cos^{2}2\pi xdx+\int_{1-h}^{1} \cos^{2}2\pi xdx\right)\right]}.$$
(49)

Tab. 9 shows the approximate frequencies obtained by the homotopy perturbation method (Eq. (47)) and Galerkin method with CA and WLA (Eq. (49)), and compares those with exact frequency (Eq. (48)).

Table 9. Comparison of approximate frequencies with exact frequency

$A\omega_E$ [25]	$\begin{array}{c} A\omega_{wla},\\ p=0 \end{array}$	error %	$\begin{array}{l}A\omega_{wla},\\p=0.25\end{array}$	error %	$A\omega_{wla},\ p=0.5$	error %	$\begin{array}{c} A\omega_{wla},\\ p=1 \end{array}$	error %	$A\omega_{hpm}$ [16]	error %
1.2533	1.4142	12.839	1.3869	10.659	1.3611	8.601	1.3135	4.803	1.1547	7.867

It is seen from Tab. 9 that the accuracy of solution obtained by WLA for all values p = 0.25; 0.5; 1 is better than the accuracy of solution obtained by CA. The percent error of solution obtained by WLA for p = 1 is smaller than the one of solution obtained by the homotopy perturbation method, however, the percent errors of solution obtained by WLA for p = 0.25 and p = 0.5 are bigger than the one of solution obtained by the homotopy perturbation method.

4.4. Duffing-harmonic oscillator

Consider a free oscillator with a rational form of the restoring force which is governed by the following equation

$$\ddot{z} + \frac{z^3}{1+z^2} = 0, \quad z(0) = A, \quad \dot{z}(0) = 0.$$
 (50)

In this case the nonlinear function takes the form

$$f(z) = \frac{z^3}{1+z^2}.$$
(51)

It is interesting to note that for small z, Eq. (50) is closed to the Duffing nonlinear oscillator, while for the large z, the equation approximates a linear oscillator; For this reason the system (50) is called the Duffing-harmonic oscillator.

The investigation of this nonlinear oscillator was started by Mickens [26] and then continued by many authors [27–32]. Substituting Eq. (51) into Eq. (33) leads to the following approximate frequency for the Duffing-harmonic oscillator (50)

$$\frac{\omega_{wla}^{2} = \frac{(1-2ph)\int_{0}^{1} \frac{(A\cos 2\pi x)^{3}}{1+(A\cos 2\pi x)^{2}}\cos 2\pi x dx + p\left(\int_{0}^{h} \frac{(A\cos 2\pi x)^{3}}{1+(A\cos 2\pi x)^{2}}\cos 2\pi x dx + \int_{1-h}^{1} \frac{(A\cos 2\pi x)^{3}}{1+(A\cos 2\pi x)^{2}}\cos 2\pi x dx\right)}{(1-2ph)\int_{0}^{1} A\cos^{2} 2\pi x dx + p\left(\int_{0}^{h} A\cos^{2} 2\pi x dx + \int_{1-h}^{1} A\cos^{2} 2\pi x dx\right)}.$$
(52)

The exact frequency is presented in [29] as follows

$$\omega_E = \frac{\pi}{2} \left(\int_{0}^{\pi/2} \frac{A \cos t dt}{\sqrt{A^2 \cos^2 t + \ln\left[1 - A^2 \cos^2 t/(1 + A^2)\right]}} \right)^{-1}.$$
(53)

The approximate frequency for the system (50) obtained by the parameter-expansion method (PEM) is shown in [33]

$$\omega_{PEM} = \sqrt{\frac{3A^2}{4+3A^2}}.$$
(54)

Table 10. Comparison of approximate frequencies with exact frequency

А	ω _E [29]	$egin{array}{l} \omega_{wla},\ p=0 \end{array}$	error %	$\omega_{wla},\ p=0.25$	error %	$\omega_{wla},\ p=0.5$	error %	$egin{array}{l} \omega_{wla},\ p=1 \end{array}$	error %	ω _{ΡΕΜ} [33]	error %
0.1	0.08349	0.08624	3.299	0.08548	2.377	0.08464	1.378	0.08274	0.903	0.08628	3.342
0.5	0.38737	0.39423	1.771	0.39108	0.957	0.38766	0.074	0.37990	1.942	0.39736	2.579
1	0.63678	0.64359	1.070	0.63962	0.445	0.63530	0.233	0.62543	1.783	0.65465	2.807

Tab. 10 shows the approximate frequencies obtained by PEM (Eq. (54)) and Galerkin method with CA and WLA (Eq. (52)), and compares those with exact frequency (Eq. (53)). It is seen from Tab. 10 that the accuracy of solution obtained by WLA for p = 0.25; 0.5 is better than the accuracy of solution obtained by WLA for p = 0.25; 0.5 is better than the accuracy of solution obtained by WLA for all values p = 0.25; 0.5; 1 are smaller than the ones of solution obtained by the parameter-expansion method.

5. CONCLUSIONS

The averaged values play a key role in many areas of science and engineering hence an extension to these values is presented. For a function given a simple form of weighted local averaging operator (WLAO) taking into account the particular role of boundary values of the function is constructed. Remarkable features of WLAO was that it contains a parameter of boundary regulation p and depends on a local value h of the integration domain, and WLAO coincides with conventional averaging operator (CAO) at three specific values of h, namely h = 0; 0.5 and 1. By varying these two parameters one can regulate the obtained approximate solutions in order to get more accurate ones. In particular by putting p = 0, WLAO leads to CAO. It has been shown that the connection of WLAO with Galerkin method forms an effective approximate tool for the buckling problem of columns and the frequency analysis of strongly nonlinear systems. Detailed numerical calculations are carried out with three specific values of the boundary regulation parameter, namely, p = 0.25, 0.5 and 1 for some typical columns and

strongly nonlinear systems. It is obtained that the accuracy of solutions obtained by WLA is significantly improved in comparison to the one of solutions obtained by CA, specially, when the change of the cross-section or the system nonlinearity are increased. WLA shows to be an effective tool which is sophisticate and can be supported to CA to obtain more accurate solutions. Further comprehensive investigations, however, need to be carried out in order to find appropriate values of the boundary regulation parameter p that can give most approximate solutions for large classes of problems. The key to the best accuracy of obtained solutions is of course related to the best choice of p which will likely depend on the problem to be solved, including the boundary conditions. To solve this problem the approach of the finite element analysis can be useful. It is possible for example, to distinguish the essential boundary conditions from natural boundary conditions Bathe [34], and would use a finer mesh or "overlapping finite elements" at the boundaries with natural boundary conditions Zhang and Bathe [35]. On the other hand the proposed WLA needs to be verified for buckling problems of more complicate structures such as nonlinear columns, laminated composite plates and shells Reddy [36] where the analytical use of Galerkin method is quite often limited by the first order approximation.

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